## Traffic Assessment

# Gas Station/Convenience Store 23 Central Street Byfield, Massachusetts 

## Prepared for:

## A.L. Prime Energy Consultant, Inc. 18 Lark Avenue Saugus, MA 01906

March 11, 2020


Ron Müller \& Associates

## Traffic Assessment

To: Mr. Anthony Guba
A.L. Prime Energy Consultant, Inc.

18 Lark Avenue
Saugus, MA 01906
From: Kirsten Braun, P.E., Senior Traffic Eng. Project \#: 19046

Reg: Gas Station/Convenience Store<br>23 Central Street Byfield, MA

Date: March 11, 2020
Prest

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outside the sight triangles. The proposed drive-through lane can accommodate an anticipated average maximum queue length of 11 vehicles. The maximum potential queue of 13 vehicles can also be accommodated on-site without affecting circulation or access. It is recommended that the proposed drive-through lane be clearly identified through signing and striping.

Figure 1
Site Location Map


## TRAFFIC VOLUMES AND VEHICLE SPEEDS

Traffic volume and vehicle speed information along Central Street just east of Fruit Street near the site was obtained from an automatic traffic recorder (ATR) count conducted in November 2019. To determine if the count data needed to be adjusted to represent annual average-month conditions, historical traffic volume data were obtained from the MassDOT. Based on the nearest MassDOT permanent count station located on Interstate 95, north of Topsfield (Station 5085), traffic during the month of November is approximately four percent below annual average-month conditions. Therefore, the November counts were increased by 4.0 percent. The traffic counts and MassDOT permanent count station data are provided in the Appendix. A summary of the available traffic counts is shown in Table 1. Additionally, a summary of observed travel speeds along Central Street are summarized in Table 2.

Table 1
Existing Traffic Volume Summary

| Location | Daily Volume ${ }^{\text {a }}$ | Peak Hour Volume ${ }^{\text {b }}$ |  | K-Factor ${ }^{\text {c }}$ | Directional Distribution ${ }^{\text {d }}$ |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Central Street | 7,250 | AM: | 733 | 10.1\% | 56\% EB |
| East the Site: |  | PM: | 646 | 8.9\% | 46\% EB |

${ }^{a}$ In vehicles per day.
${ }^{\mathrm{b}}$ In vehicles per hour.
${ }^{\text {c }}$ Percentage of daily traffic occurring during the peak hour.
${ }^{\mathrm{d}} \mathrm{NB}=$ northbound, $\mathrm{SB}=$ southbound.

Table 2
Observed Travel Speeds ${ }^{\text {a }}$

| Location/Direction | Posted Speed Limit | Average Speed | $\begin{gathered} 85^{\text {th }} \text { Percentile } \\ \text { Speed }^{\text {b }} \\ \hline \end{gathered}$ |
| :---: | :---: | :---: | :---: |
| Central Street East of the site |  |  |  |
|  |  |  |  |
| Eastbound | 35 | 31 | 36 |
| Westbound | 35 | 29 | 34 |

[^0]As shown, the average recorded speeds along Central Street adjacent to the site were slightly lower than the posted speed limit of 35 miles per hour ( mph ) with 31 mph eastbound and 29 mph westbound. Average speeds along this stretch of roadway may be slightly lower than the posted speed limit due to the proximity to the I- 95 ramps. The $85^{\text {th }}$ percentile speeds were recorded to be 36 mph eastbound and 34 mph westbound, comparable to the posted speed limit. $85^{\text {th }}$ percentile speeds were accordingly used in the calculation of minimum sight distance requirements, as described below.

## SIGHT DISTANCE

To identify potential safety concerns associated with site access and egress, sight distances have been evaluated at the proposed site driveways on Central Street to determine if the available sight distances for vehicles exiting the site meet or exceed the minimum distances required for approaching vehicles to safely stop. The available sight distances were compared with minimum requirements, as established by the American Association of State Highway and Transportation Officials (AASHTO). ${ }^{1}$ AASHTO is the national standard by which vehicle sight distance is calculated, measured, and reported. The Massachusetts Department of Transportation (MassDOT) and the Executive Office of Energy and Environmental Affairs (EEA) require the use of AASHTO sight distance standards when preparing traffic impact assessments and studies, as stated in their guidelines for traffic impact assessments.

Sight distance is the length of roadway ahead that is visible to the driver. Stopping Sight Distance (SSD) is the minimum distance required for a vehicle traveling at a certain speed to safely stop before reaching a stationary object in its path. The values are based on a driver perception and reaction time of 2.5 seconds and a braking distance calculated for wet, level pavements. When the roadway is either on an upgrade or downgrade, grade correction factors are applied. Stopping sight distance is measured from an eye height of 3.5 feet to an object height of 2 feet above street level, equivalent to the taillight height of a passenger car. The SSD is measured along the centerline of the traveled way of the major road.

Intersection sight distance (ISD) is provided on minor street approaches to allow the drivers of stopped vehicles a sufficient view of the major roadway to decide when to enter the major roadway. By definition, ISD is the minimum distance required for a motorist exiting a minor street to turn onto the major street, without being overtaken by an approaching vehicle reducing its speed from the design speed to 70 percent of the design speed. ISD is measured from an eye height of 3.5 feet to an object height of 3.5 feet above street level. The use of an object height equal to the driver eye height makes intersection sight distances reciprocal (i.e., if one driver can see another vehicle, then the driver of that vehicle can also see the first vehicle). When the minor street is on an upgrade that exceeds 3 percent, grade correction factors are applied.

[^1]SSD is generally more important as it represents the minimum distance required for safe stopping while ISD is based only upon acceptable speed reductions to the approaching traffic stream. However, the ISD must be equal to or greater than the minimum required SSD in order to provide safe operations at the intersection. In accordance with the AASHTO manual, "If the available sight distance for an entering or crossing vehicle is at least equal to the appropriate stopping sight distance for the major road, then drivers have sufficient sight distance to anticipate and avoid collisions. However, in some cases, this may require a major-road vehicle to stop or slow to accommodate the maneuver by a minor-road vehicle. To enhance traffic operations, intersection sight distances that exceed stopping sight distances are desirable along the major road." Accordingly, ISD should be at least equal to the distance required to allow a driver approaching the minor road to safely stop.

The available sight distances at the proposed driveways on Central Street were measured and compared to minimum requirements as established by AASHTO. On Central Street, the $85^{\text {th }}$ percentile speeds were used over the posted speed limit of 35 mph to determine minimum required sight distance. The required minimum sight distances are compared to the available distances, as shown in Table 3.

Table 3
Sight Distance Summary

| Location/Direction | Intersection Sight Distance (feet) |  |  |
| :---: | :---: | :---: | :---: |
|  | Measured | Minimum Required ${ }^{\text {a }}$ | Desirable ${ }^{\text {b }}$ |
| Central St. at East Site Driveway: |  |  |  |
| East of intersection | 410 | 250 | 390 |
| West of Intersection | 500+ | 261 | 390 |
| Central St. at West Site Driveway: |  |  |  |
| East of intersection | 490 | 250 | 390 |
| West of Intersection | $500+$ | 261 | 390 |

${ }^{\text {a }}$ Values based on AASHTO SSD requirements for the $85^{\text {th }}$ percentile speed of 36 mph on Central Street traveling eastbound and 35 mph posted speed limit westbound.
${ }^{\mathrm{b}}$ Values based on AASHTO ISD requirements for the posted speed limit of 35 mph on Central Street.

As shown in the table, both the minimum required and desirable sight distances are exceeded at the east and west site driveways on Central Street and safe operation of these driveways can therefore be expected. To ensure that maximum sight distances are maintained, it is recommended that any proposed landscaping or signs in the vicinity of the driveways be kept low (maximum two feet in height from street level), or set back sufficiently so as not to impede the available sight distances.

## SITE ACCESS, CIRCULATION AND QUEUING

Access to the site is proposed via two new curb cuts on Central Street. Both driveways are proposed to be 30 feet-wide which allows for less mountable curbing and pavement outside of the defined driveways to accommodate delivery-vehicle access to/from the site, in particular fuel deliveries. The site plan includes a swept path analysis showing how fuel delivery vehicles can navigate the site and driveways under this design.

The drive-through lane will provide approximately 220 feet of storage allowing for a total of 11 cars to be queued at the drive-through window. Beyond the drive-through lane, an additional 6 cars could be queued on site before affecting site access. The roadway around the building with the drive-through queue will be a minimum of 24 feet wide allowing more than adequate room for a vehicle to by-pass the drive-through queue. Although there are parking spaces located in the rear of the site adjacent to the drive-through lane, these spaces will be restricted to employee parking so as to minimize any conflict with the drive-through operation.

Based on a published study ${ }^{2}$ of drive-through queuing at a number of different land uses, the average maximum queue at coffee shops was found to be 11 vehicles and the $85^{\text {th }}$ percentile maximum queue was 13 vehicles. The study was performed at six different coffee shops including four Starbucks and two Caribou coffee shops located in Minnesota and Kansas. More locally, drive-through queue studies have been performed at four different Dunkin' Donuts coffee shops located in Malden, Kingston, and Dracut, Massachusetts and in Milford, New Hampshire. The maximum observed queue at these locations was 13 vehicles during the weekday AM peak hour, six vehicles during the weekday PM peak hour, and 10 vehicles during the Saturday peak hour. A summary of the queue studies is provided in the Appendix.

Based on these studies, there is adequate queuing space to accommodate the average maximum queue of 11 vehicles. The maximum queue of 13 vehicles observed at any one site can also easily be accommodated on site without affecting on site circulation or site access. The drive-through lane should be clearly marked through signing and striping including DRIVE THRU pavement markings with pavement arrows.

[^2]
## TRIP GENERATION

The traffic to be generated by the proposed gas station and convenience store was estimated using the $10^{\text {th }}$ Edition of the Institute of Transportation Engineers (ITE) Trip Generation Manual ${ }^{3}$. The site plan prepared by A.L. Prime Energy Consultant, Inc. shows 12 fueling positions on site as well as a 4,850 square-foot convenience store with coffee/donut shop. Accordingly, ITE Land Use Code 960 (Super Convenience Market/Gas Station) was used in estimating the traffic generation characteristics of the project, as shown in Table 4. The trip generation calculations are provided in the Appendix.

Table 4
Trip Generation Summary

| Time Period | Total Trips ${ }^{\text {a }}$ | Pass-by Trips ${ }^{\text {b }}$ | New Trips |
| :---: | :---: | :---: | :---: |
| Weekday Daily | 2,770 | 1,550 | 1,220 |
| Weekday AM Peak Hour |  |  |  |
| Enter | 169 | 105 | 64 |
| Exit | $\underline{168}$ | $\underline{105}$ | 63 |
| Total | 337 | 210 | 127 |
| Weekday PM Peak Hour |  |  |  |
| Enter | 138 | 77 | 61 |
| Exit | 138 | 77 | 61 |
| Total | 276 | 154 | 122 |

${ }^{\text {a }}$ ITE Land Use Code 960 (Super Convenience Market/Gas Station) for 12 fueling positions.
${ }^{\text {b }}$ ITE Trip Generation Handbook. Pass-by rate of $62 \%$ applied to weekday AM total trips and a pass-by rate of $56 \%$ applied to weekday daily and PM total trips.

As shown in Table 4, the gas station and convenience store on Central Street is expected to generate 337 vehicle trips ( 169 in and 168 out) during the weekday AM peak hour and 276 vehicle trips (138 in and 138 out) during the weekday PM peak hour. Not all vehicle trips generated by the project, however, represent new trips. Studies have shown that gas stations with convenience stores generate more than half of their business from the traffic already present on the adjacent roadway. This traffic is referred to as pass-by trips. Based on data published in the ITE Trip Generation Handbook, ${ }^{4}$ an average of 56 to 62 percent of the total traffic generated by gas stations with convenience stores is typically pass-by traffic, depending on the time period. Therefore, while the

[^3]total traffic generated by the project will be realized at the site driveways, the impact of that traffic on the adjacent streets is substantially less.

As shown in Table 5, with the influence of pass-by traffic, the actual volume of new traffic to be added to the adjacent streets is 127 trips during the weekday AM peak hour ( 64 entering and 63 exiting) and 122 trips during the weekday PM peak hour (61 entering and 61 exiting).

## TRIP DISTRIBUTION

The distribution of new site traffic on the area roadways was based on population densities near the site as well as access from the regional highway network. Accordingly, approximately 70percent of site traffic is expected on Central Street to and from the west with at least 40 percent to/from I-95 and 30-percent to and from the east. The distribution of pass-by traffic was based on existing travel patterns on Central Street.

## TRAFFIC INCREASES

The proposed development project will result in increases in traffic on the study area roadways. Traffic-volume increases on Central Street to the east of the site and west of I-95 are expected in the range of 37 to 38 vehicles during the peak hours, or an average of one additional vehicle every $11 / 2$ to two minutes. This additional traffic represents an increase of 5 to 6 percent over existing Central Street traffic. The largest increase in traffic from the development project will occur on the short section of Central Street between the site and I-95, with 86 to 89 additional vehicles during the peak hours, or an average of one to two additional vehicles every minute.

## CONCLUSIONS

- The project entails the development of a 12 -vehicle fueling position gas station and 4,850 square foot convenience store with coffee/donut shop and drive through window at 23 Central Street in Byfield, Massachusetts. Site access is proposed via two new curb cuts to be constructed on Central Street.
- The proposed gas station with convenience store and coffee shop is expected to generate an additional 337 vehicle trips during the weekday AM peak hour (169 entering and 168
exiting) and 276 vehicle trips during the weekday PM peak hour (138 entering and 138 exiting). These additional trips will be realized at the site driveway.
- More than half of these trips will already be present on the adjacent roadways and will not be new to the area. These trips are referred to as pass-by trips. Accordingly, the increase in traffic on Central Street east of the site and west of I-95 is expected in the range of 37 to 38 vehicles during the peak hours, or an average of one additional vehicle every $1 \frac{1}{2}$ to two minutes. This additional traffic represents an increase of 5 to 6 percent over existing Central Street traffic.
- In the short section of Central Street between the site and I-95, an increase in traffic of 86 to 89 additional vehicles is expected during the peak hours, averaging about one to two additional vehicles every minute during both the AM and PM peak hours.
- Two 30-foot wide driveways with mountable curbing are proposed to accommodate larger delivery vehicles entering the site.
- Drive-through queue studies at coffee/donut shops have shown an average maximum queue length of 11 vehicles with the maximum queue at any one site observed at 13 vehicles. Based on the latest site plan the drive-through will be able to accommodate 11 vehicles. Beyond this drive-through lane, there is more than adequate room on site to accommodate a maximum potential queue of 13 vehicles without affecting on-site circulation and access. It is recommended that the drive-through lane be clearly marked through signing and striping including DRIVE THRU pavement markings with pavement arrows.
- The minimum required sight distances as well as the desired sight distances are exceeded at both site driveways on Central Street and therefore safe operation can be expected.
- It is recommended that any proposed landscaping or signs in the vicinity of the driveways be kept low to the ground (less than two feet above street level) or set back sufficiently so as not to impede sight distances for drivers exiting the site.


## APPENDIX

Traffic Counts and Vehicle Speed Data
Seasonal Adjustment Data
Drive-Through Queue Studies
Trip Generation Calculations

Location : Central Street
Location : East of Fruit Street
City/State: Byfield, MA

| Start | 11/20/201 | WB |  | Hour Totals |  | EB |  | Hour Totals |  | Combined Totals |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Time | Wed | Morning | Afternoon | Morning | Afternoon | Morning | Afternoon | Morning | Afternoon | Morning | Afternoon |
| 12:00 |  | 1 | 53 |  |  | 2 | 39 |  |  |  |  |
| 12:15 |  | 0 | 47 |  |  | 2 | 42 |  |  |  |  |
| 12:30 |  | 0 | 48 |  |  | 2 | 42 |  |  |  |  |
| 12:45 |  | 0 | 34 | 1 | 182 | 3 | 43 | 9 | 166 | 10 | 348 |
| 01:00 |  | 1 | 49 |  |  | 4 | 27 |  |  |  |  |
| 01:15 |  | 0 | 47 |  |  | 1 | 39 |  |  |  |  |
| 01:30 |  | 0 | 41 |  |  | 1 | 41 |  |  |  |  |
| 01:45 |  | 0 | 52 | 1 | 189 | 1 | 66 | 7 | 173 | 8 | 362 |
| 02:00 |  | 4 | 52 |  |  | 0 | 50 |  |  |  |  |
| 02:15 |  | 1 | 110 |  |  | 0 | 66 |  |  |  |  |
| 02:30 |  | 0 | 81 |  |  | 0 | 56 |  |  |  |  |
| 02:45 |  | 0 | 65 | 5 | 308 | 1 | 58 | 1 | 230 | 6 | 538 |
| 03:00 |  | 2 | 68 |  |  | 0 | 83 |  |  |  |  |
| 03:15 |  | 2 | 67 |  |  | 2 | 64 |  |  |  |  |
| 03:30 |  | 1 | 94 |  |  | 4 | 69 |  |  |  |  |
| 03:45 |  | 0 | 73 | 5 | 302 | 0 | 70 | 6 | 286 | 11 | 588 |
| 04:00 |  | 1 | 63 |  |  | 0 | 87 |  |  |  |  |
| 04:15 |  | 2 | 73 |  |  | 3 | 78 |  |  |  |  |
| 04:30 |  | 4 | 95 |  |  | 1 | 62 |  |  |  |  |
| 04:45 |  | 9 | 71 | 16 | 302 | 2 | 65 | 6 | 292 | 22 | 594 |
| 05:00 |  | 18 | 90 |  |  | 5 | 76 |  |  |  |  |
| 05:15 |  | 18 | 84 |  |  | 13 | 65 |  |  |  |  |
| 05:30 |  | 27 | 83 |  |  | 13 | 71 |  |  |  |  |
| 05:45 |  | 22 | 53 | 85 | 310 | 13 | 70 | 44 | 282 | 129 | 592 |
| 06:00 |  | 43 | 47 |  |  | 22 | 57 |  |  |  |  |
| 06:15 |  | 43 | 50 |  |  | 32 | 46 |  |  |  |  |
| 06:30 |  | 48 | 51 |  |  | 50 | 60 |  |  |  |  |
| 06:45 |  | 50 | 28 | 184 | 176 | 67 | 48 | 171 | 211 | 355 | 387 |
| 07:00 |  | 68 | 34 |  |  | 100 | 23 |  |  |  |  |
| 07:15 |  | 96 | 24 |  |  | 138 | 32 |  |  |  |  |
| 07:30 |  | 94 | 19 |  |  | 101 | 36 |  |  |  |  |
| 07:45 |  | 72 | 13 | 330 | 90 | 60 | 32 | 399 | 123 | 729 | 213 |
| 08:00 |  | 88 | 21 |  |  | 48 | 33 |  |  |  |  |
| 08:15 |  | 69 | 20 |  |  | 62 | 25 |  |  |  |  |
| 08:30 |  | 52 | 12 |  |  | 52 | 32 |  |  |  |  |
| 08:45 |  | 59 | 21 | 268 | 74 | 52 | 17 | 214 | 107 | 482 | 181 |
| 09:00 |  | 48 | 8 |  |  | 36 | 20 |  |  |  |  |
| 09:15 |  | 49 | 9 |  |  | 30 | 18 |  |  |  |  |
| 09:30 |  | 40 | 6 |  |  | 42 | 13 |  |  |  |  |
| 09:45 |  | 44 | 11 | 181 | 34 | 31 | 12 | 139 | 63 | 320 | 97 |
| 10:00 |  | 44 | 10 |  |  | 42 | 10 |  |  |  |  |
| 10:15 |  | 64 | 3 |  |  | 35 | 8 |  |  |  |  |
| 10:30 |  | 59 | 2 |  |  | 44 | 2 |  |  |  |  |
| 10:45 |  | 39 | 6 | 206 | 21 | 31 | 4 | 152 | 24 | 358 | 45 |
| 11:00 |  | 39 | 3 |  |  | 27 | 8 |  |  |  |  |
| 11:15 |  | 41 | 5 |  |  | 37 | 11 |  |  |  |  |
| 11:30 |  | 44 | 0 |  |  | 35 | 5 |  |  |  |  |
| 11:45 |  | 38 | 1 | 162 | 9 | 43 | 3 | 142 | 27 | 304 | 36 |
| Total |  | 1444 | 1997 |  |  | 1290 | 1984 |  |  | 2734 | 3981 |
| Percent |  | 42.0\% | 58.0\% |  |  | 39.4\% | 60.6\% |  |  | 40.7\% | 59.3\% |

Location : Central Street
Location : East of Fruit Street
City/State: Byfield, MA
19024VL1

| Start | 11/21/201 | WB |  | Hour Totals |  | EB |  | Hour Totals |  | Combined Totals |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Time | Thu | Morning | Afternoon | Morning | Afternoon | Morning | Afternoon | Morning | Afternoon | Morning | Afternoon |
| 12:00 |  | 0 | 44 |  |  | 5 | 38 |  |  |  |  |
| 12:15 |  | 0 | 41 |  |  | 3 | 35 |  |  |  |  |
| 12:30 |  | 1 | 48 |  |  | 1 | 39 |  |  |  |  |
| 12:45 |  | 0 | 32 | 1 | 165 | 2 | 51 | 11 | 163 | 12 | 328 |
| 01:00 |  | 0 | 36 |  |  | 2 | 44 |  |  |  |  |
| 01:15 |  | 1 | 47 |  |  | 2 | 41 |  |  |  |  |
| 01:30 |  | 1 | 54 |  |  | 0 | 36 |  |  |  |  |
| 01:45 |  | 0 | 45 | 2 | 182 | 0 | 64 | 4 | 185 | 6 | 367 |
| 02:00 |  | 1 | 54 |  |  | 1 | 56 |  |  |  |  |
| 02:15 |  | 0 | 101 |  |  | 1 | 73 |  |  |  |  |
| 02:30 |  | 0 | 84 |  |  | 1 | 46 |  |  |  |  |
| 02:45 |  | 0 | 70 | 1 | 309 | 0 | 64 | 3 | 239 | 4 | 548 |
| 03:00 |  | 0 | 84 |  |  | 1 | 71 |  |  |  |  |
| 03:15 |  | 5 | 84 |  |  | 1 | 77 |  |  |  |  |
| 03:30 |  | 3 | 110 |  |  | 2 | 59 |  |  |  |  |
| 03:45 |  | 0 | 88 | 8 | 366 | 3 | 73 | 7 | 280 | 15 | 646 |
| 04:00 |  | 2 | 91 |  |  | 0 | 70 |  |  |  |  |
| 04:15 |  | 3 | 85 |  |  | 3 | 60 |  |  |  |  |
| 04:30 |  | 2 | 84 |  |  | 0 | 68 |  |  |  |  |
| 04:45 |  | 13 | 74 | 20 | 334 | 6 | 62 | 9 | 260 | 29 | 594 |
| 05:00 |  | 15 | 113 |  |  | 6 | 60 |  |  |  |  |
| 05:15 |  | 23 | 103 |  |  | 4 | 70 |  |  |  |  |
| 05:30 |  | 25 | 84 |  |  | 13 | 83 |  |  |  |  |
| 05:45 |  | 20 | 56 | 83 | 356 | 28 | 80 | 51 | 293 | 134 | 649 |
| 06:00 |  | 48 | 59 |  |  | 20 | 63 |  |  |  |  |
| 06:15 |  | 30 | 41 |  |  | 27 | 47 |  |  |  |  |
| 06:30 |  | 57 | 26 |  |  | 41 | 54 |  |  |  |  |
| 06:45 |  | 62 | 31 | 197 | 157 | 69 | 57 | 157 | 221 | 354 | 378 |
| 07:00 |  | 55 | 33 |  |  | 79 | 49 |  |  |  |  |
| 07:15 |  | 62 | 25 |  |  | 129 | 42 |  |  |  |  |
| 07:30 |  | 111 | 32 |  |  | 89 | 65 |  |  |  |  |
| 07:45 |  | 62 | 25 | 290 | 115 | 94 | 29 | 391 | 185 | 681 | 300 |
| 08:00 |  | 73 | 16 |  |  | 113 | 34 |  |  |  |  |
| 08:15 |  | 61 | 25 |  |  | 74 | 40 |  |  |  |  |
| 08:30 |  | 67 | 20 |  |  | 55 | 30 |  |  |  |  |
| 08:45 |  | 58 | 16 | 259 | 77 | 65 | 31 | 307 | 135 | 566 | 212 |
| 09:00 |  | 51 | 20 |  |  | 52 | 20 |  |  |  |  |
| 09:15 |  | 57 | 14 |  |  | 53 | 33 |  |  |  |  |
| 09:30 |  | 55 | 16 |  |  | 45 | 20 |  |  |  |  |
| 09:45 |  | 63 | 30 | 226 | 80 | 40 | 17 | 190 | 90 | 416 | 170 |
| 10:00 |  | 65 | 8 |  |  | 33 | 13 |  |  |  |  |
| 10:15 |  | 47 | 8 |  |  | 36 | 9 |  |  |  |  |
| 10:30 |  | 37 | 5 |  |  | 29 | 9 |  |  |  |  |
| 10:45 |  | 44 | 16 | 193 | 37 | 47 | 8 | 145 | 39 | 338 | 76 |
| 11:00 |  | 42 | 5 |  |  | 41 | 7 |  |  |  |  |
| 11:15 |  | 51 | 6 |  |  | 39 | 7 |  |  |  |  |
| 11:30 |  | 53 | 1 |  |  | 45 | 5 |  |  |  |  |
| 11:45 |  | 46 | 3 | 192 | 15 | 40 | 7 | 165 | 26 | 357 | 41 |
| Total |  | 1472 | 2193 |  |  | 1440 | 2116 |  |  | 2912 | 4309 |
| Percent |  | 40.2\% | 59.8\% |  |  | 40.5\% | 59.5\% |  |  | 40.3\% | 59.7\% |
| Grand Total |  | 2916 | 4190 |  |  | 2730 | 4100 |  |  | 5646 | 8290 |
| Percent |  | 41.0\% | 59.0\% |  |  | 40.0\% | 60.0\% |  |  | 40.5\% | 59.5\% |
| ADT |  | DT 6,968 |  | DT 6,968 |  |  |  |  |  |  |  |


| Start | 11/18/2019 |  |  | Tue |  | Wed |  | Thu |  | Fri |  | Sat |  | Sun |  | Week Average |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Time | WB |  | EB | WB | EB | WB | EB | WB | EB | WB | EB | WB | EB | WB | EB | WB | EB |
| 12:00 AM |  | * | * | * | * | 1 | 9 | 1 | 11 | * | * | * | * | * | * | 1 | 10 |
| 01:00 |  | * | * | * | * | 1 | 7 | 2 | 4 | * | * | * | * | * | * | 2 | 6 |
| 02:00 |  | * | * | * | * | 5 | 1 | 1 | 3 | * | * | * | * | * | * | 3 | 2 |
| 03:00 |  | * | * | * | * | 5 | 6 | 8 | 7 | * | * | * | * | * | * | 6 | 6 |
| 04:00 |  | * | * | * | * | 16 | 6 | 20 | 9 | * | * | * | * | * | * | 18 | 8 |
| 05:00 |  | * | * | * | * | 85 | 44 | 83 | 51 | * | * | * | * | * | * | 84 | 48 |
| 06:00 |  | * | * | * | * | 184 | 171 | 197 | 157 | * | * | * | * | * | * | 190 | 164 |
| 07:00 |  | * | * | * | * | 330 | 399 | 290 | 391 | * | * | * | * | * | * | 310 | 395 |
| 08:00 |  | * | * | * | * | 268 | 214 | 259 | 307 | * | * | * | * | * | * | 264 | 260 |
| 09:00 |  | * | * | * | * | 181 | 139 | 226 | 190 | * | * | * | * | * | * | 204 | 164 |
| 10:00 |  | * | * | * | * | 206 | 152 | 193 | 145 | * | * | * | * | * | * | 200 | 148 |
| 11:00 |  | * | * | * | * | 162 | 142 | 192 | 165 | * | * | * | * | * | * | 177 | 154 |
| 12:00 PM |  | * | * | * | * | 182 | 166 | 165 | 163 | * | * | * | * | * | * | 174 | 164 |
| 01:00 |  | * | * | * | * | 189 | 173 | 182 | 185 | * | * | * | * | * | * | 186 | 179 |
| 02:00 |  | * | * | * | * | 308 | 230 | 309 | 239 | * | * | * | * | * | * | 308 | 234 |
| 03:00 |  | * | * | * | * | 302 | 286 | 366 | 280 | * | * | * | * | * | * | 334 | 283 |
| 04:00 |  | * | * | * | * | 302 | 292 | 334 | 260 | * | * | * | * | * | * | 318 | 276 |
| 05:00 |  | * | * | * | * | 310 | 282 | 356 | 293 | * | * | * | * | * | * | 333 | 288 |
| 06:00 |  | * | * | * | * | 176 | 211 | 157 | 221 | * | * | * | * | * | * | 166 | 216 |
| 07:00 |  | * | * | * | * | 90 | 123 | 115 | 185 | * | * | * | * | * | * | 102 | 154 |
| 08:00 |  | * | * | * | * | 74 | 107 | 77 | 135 | * | * | * | * | * | * | 76 | 121 |
| 09:00 |  | * | * | * | * | 34 | 63 | 80 | 90 | * | * | * | * | * | * | 57 | 76 |
| 10:00 |  | * | * | * | * | 21 | 24 | 37 | 39 | * | * | * | * | * | * | 29 | 32 |
| 11:00 |  | * | * | * | * | 9 | 27 | 15 | 26 | * | * | * | * | * | * | 12 | 26 |
| Lane | 0 | 0 | 0 | 0 | 0 | 3441 | 3274 | 3665 | 3556 | 0 | 0 | 0 | 0 | 0 | 0 | 3554 | 3414 |
| Day |  | 0 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| AM Peak |  | - | - | - | - | 07:00 | 07:00 | 07:00 | 07:00 | - | - | - | - | - | - | 07:00 | 07:00 |
| Vol. |  | - | - | - | - | 330 | 399 | 290 | 391 | - | - | - | - | - | - | 310 | 395 |
| PM Peak |  | - | - | - | - | 17:00 | 16:00 | 15:00 | 17:00 | - | - | - | - | - | - | 15:00 | 17:00 |
| Vol. |  | - | - | - | - | 310 | 292 | 366 | 293 | - | - | - | - | - | - | 334 | 288 |


Location: Central Street
Location : East of Fruit Street
City/State: Byfield, MA
$\begin{array}{rrrrrr}\text { PM Peak } & - & - & - & - & 17.00 \\ \text { Vol. } & - & - & - & - & 310\end{array}$

| Start | 1 | 16 | 21 | 26 | 31 | 36 | 41 | 46 | 51 | 56 | 61 | 66 | 71 | 76 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Time | 15 | 20 | 25 | 30 | 35 | 40 | 45 | 50 | 55 | 60 | 65 | 70 | 75 | 999 | Total |
| 11/20/19 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 |
| 01:00 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 |
| 02:00 | 1 | 0 | 1 | 0 | 2 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 5 |
| 03:00 | 0 | 0 | 1 | 3 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 5 |
| 04:00 | 0 | 0 | 3 | 3 | 6 | 3 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 16 |
| 05:00 | 0 | 2 | 3 | 19 | 43 | 14 | 4 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 85 |
| 06:00 | 0 | 6 | 14 | 47 | 81 | 31 | 4 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 184 |
| 07:00 | 0 | 9 | 47 | 92 | 129 | 48 | 5 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 330 |
| 08:00 | 0 | 7 | 23 | 76 | 120 | 38 | 3 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 268 |
| 09:00 | 0 | 6 | 20 | 65 | 72 | 15 | 3 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 181 |
| 10:00 | 1 | 7 | 29 | 60 | 88 | 14 | 6 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 206 |
| 11:00 | 2 | 3 | 24 | 50 | 64 | 15 | 4 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 162 |
| 12 PM | 1 | 7 | 21 | 63 | 65 | 25 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 182 |
| 13:00 | 0 | 12 | 19 | 69 | 62 | 23 | 4 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 189 |
| 14:00 | 0 | 14 | 47 | 109 | 104 | 29 | 5 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 308 |
| 15:00 | 0 | 11 | 35 | 105 | 93 | 49 | 8 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 302 |
| 16:00 | 1 | 11 | 59 | 117 | 78 | 30 | 6 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 302 |
| 17:00 | 1 | 3 | 55 | 146 | 86 | 18 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 310 |
| 18:00 | 1 | 8 | 25 | 80 | 44 | 17 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 176 |
| 19:00 | 0 | 2 | 14 | 38 | 30 | 4 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 90 |
| 20:00 | 0 | 2 | 17 | 33 | 18 | 4 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 74 |
| 21:00 | 0 | 1 | 11 | 10 | 11 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 34 |
| 22:00 | 0 | 0 | 2 | 11 | 7 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 21 |
| 23:00 | 0 | 1 | 1 | 4 | 1 | 1 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 9 |
| Total | 8 | 113 | 471 | 1200 | 1205 | 380 | 59 | 5 | 0 | 0 | 0 | 0 | 0 | 0 | 3441 |

24 MPH
29 MPH
34 MPH
38 MPH
30 MPH
$26-35 \mathrm{MPH}$
2405
$69.9 \%$
1649
$47.9 \%$
 Location: Central Street
Location: East of Fruit Street
City/State: Byfield, MA


NB $\qquad$ K!!é


| Start | 1 | 16 | 21 | 26 | 31 | 36 | 41 | 46 | 51 | 56 | 61 | 66 | 71 | 76 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Time | 15 | 20 | 25 | 30 | 35 | 40 | 45 | 50 | 55 | 60 | 65 | 70 | 75 | 999 | Total |
| 11/20/19 | 0 | 0 | 0 | 4 | 5 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 9 |
| 01:00 | 0 | 2 | 0 | 1 | 3 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 7 |
| 02:00 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 |
| 03:00 | 0 | 0 | 1 | 1 | 3 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 6 |
| 04:00 | 0 | 0 | 2 | 2 | 1 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 6 |
| 05:00 | 0 | 0 | 2 | 16 | 23 | 3 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 44 |
| 06:00 | 0 | 1 | 8 | 42 | 83 | 33 | 3 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 171 |
| 07:00 | 3 | 1 | 19 | 90 | 194 | 79 | 11 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 399 |
| 08:00 | 2 | 4 | 15 | 48 | 96 | 40 | 7 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 214 |
| 09:00 | 0 | 5 | 4 | 38 | 62 | 27 | 3 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 139 |
| 10:00 | 6 | 8 | 17 | 29 | 59 | 28 | 3 | 1 | 1 | 0 | 0 | 0 | 0 | 0 | 152 |
| 11:00 | 1 | 1 | 15 | 39 | 65 | 20 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 142 |
| 12 PM | 0 | 3 | 20 | 42 | 66 | 28 | 7 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 166 |
| 13:00 | 0 | 7 | 18 | 48 | 64 | 31 | 4 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 173 |
| 14:00 | 5 | 8 | 20 | 76 | 86 | 30 | 4 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 230 |
| 15:00 | 2 | 4 | 33 | 86 | 110 | 50 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 286 |
| 16:00 | 4 | 10 | 36 | 90 | 121 | 26 | 4 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 292 |
| 17:00 | 0 | 11 | 32 | 94 | 109 | 33 | 3 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 282 |
| 18:00 | 0 | 7 | 19 | 96 | 70 | 19 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 211 |
| 19:00 | 0 | 4 | 12 | 55 | 47 | 4 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 123 |
| 20:00 | 0 | 6 | 17 | 34 | 42 | 7 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 107 |
| 21:00 | 0 | 1 | 5 | 23 | 28 | 6 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 63 |
| 22:00 | 0 | 0 | 4 | 8 | 8 | 3 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 24 |
| 23:00 | 0 | 1 | 2 | 10 | 10 | 3 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 27 |
| Total | 23 | 84 | 301 | 973 | 1355 | 472 | 54 | 9 | 1 | 1 | 0 | 1 | 0 | 0 | 3274 |

[^4]


ผิ



[^5]31 MPH
35 MPH
4733
$70.5 \%$
3542
$52.7 \%$
อิ.
ผิ

Mean Speed(Average)
10 MPH Pace Speed:


 Location ：Central Street
Location ：East of Fruit St Location ：East of Fruit Street
City／State：Byfield，MA WB，EB Start
ime
1／21／1
01：00
02：00
03：00
04：00
05：00
06：00
07：00
08：00
09：00
10：00
$11: 00$
12 P
$13: 00$
14：00
15：00
16：00
17：0
18：0
19：00
20：00
$21: 00$
$22: 00$
$23: 00$

Tot $\begin{array}{rr} & \\ \text { 15th Percentile ：} & 25 \mathrm{MPH} \\ \text { 50th Percentile ：} & 30 \mathrm{MPH} \\ \text { 85th Percentile ：} & 35 \mathrm{MPH} \\ \text { 95th Percentile ：} & 39 \mathrm{MPH} \\ \text { Mean Speed（Average）：} & 31 \mathrm{MPH} \\ 10 \mathrm{MPH} \text { Pace Speed ：} & 26-35 \mathrm{MPH} \\ \text { Number in Pace ：} & 5026 \\ \text { Percent in Pace ：} & 69.6 \% \\ \text { Number of Vehicles＞30 MPH ：} & 4059 \\ \text { Percent of Vehicles＞30 MPH ：} & 56.2 \%\end{array}$
Massachusetts Highway Department

| Factor Group | Station | Weight | Jan | Feb | Mar | Apr | May | Jun | Jul | Aug | Sep | Oct | Nov | Dec |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| U1-Essex | 5085 |  | 0.862 | 0.891 | 0.878 | 0.972 | 1.039 | 1.098 | 1.149 | 1.151 | 1.072 | 1.044 | 0.963 | 0.938 |
|  | Average of Weighted Factors |  | 0.862 | 0.891 | 0.878 | 0.972 | 1.039 | 1.098 | 1.149 | 1.151 | 1.072 | 1.044 | 0.963 | 0.938 |

## Observed Vehicle Queues at Dunkin Donut Drive-Through Lanes

| Peak Hour/Queue | $\begin{gathered} \text { Sep-97 } \\ \text { Malden, MA } \end{gathered}$ | $\begin{gathered} \text { May-02 } \\ \text { Milford, } \mathrm{NH}^{\mathrm{b}} \end{gathered}$ | Sep-10 <br> Kingston, MA ${ }^{\text {c }}$ | $\begin{gathered} \text { Jul-16 } \\ \text { Dracut, } \text { MA }^{\mathrm{d}} \end{gathered}$ | Average | Maximum |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Weekday AM: |  |  |  |  |  |  |
| Maximum | 11 | 11 | 11 | 13 | 12 | 13 |
| Average | 9 | 4 | 5 | 6 | 6 | 9 |
| Weekday PM: |  |  |  |  |  |  |
| Maximum | 6 | 3 | 4 | -- | 4 | 6 |
| Average | 4 | 1 | 1 | -- | 2 | 4 |
| Saturday Midday: |  |  |  |  |  |  |
| Maximum | 8 | 10 | -- | -- | 9 | 10 |
| Average | 6 | 2 | -- | -- | 4 | 6 |

${ }^{\text {a }}$ Contains a Dunkin' Donuts with a drive-through window located in Adams Plaza in Malden, MA.
${ }^{\mathrm{b}}$ Contains a Dunkin' Donuts with a drive-through window located at 143 Elm Street in Milford, NH.
${ }^{\text {c }}$ Contains a Dunkin Donuts with a drive-through window located on Routes 53/3A (Summer Street) in Kingston, MA.
${ }^{\text {d }}$ Contains a Dunkin Donuts with a drive-through window located at 177 Broadway Road in Dracut, MA.

Institute of Transportation Engineers (ITE); 10th Edition Land Use Code (LUC) 960 - Super Convenience Market/Gas Sation

Average Vehicle Trips Ends vs: Vehicle Fueling Positions
Independent Variable (X): 12

## Average Weekday Daily

$\mathrm{T}=230.52 *(\mathrm{X})$
$\mathrm{T}=2,766.24$
$\mathrm{T}=2,770 \quad$ vehicle trips
with $50 \%(1,385 \mathrm{vpd})$ entering and $50 \%(1,385 \mathrm{vpd})$ exiting.
Weekday Morning Peak Hour Of Adjacent Street Traffic
$\mathrm{T}=28.08 *(\mathrm{X})$
$\mathrm{T}=336.96$
$\mathrm{T}=337 \quad$ vehicle trips
with $50 \%(169 \mathrm{vph})$ entering and $50 \%(168 \mathrm{vph})$ exiting.

## Weekday Evening Peak Hour Of Adjacent Street Traffic

$\mathrm{T}=22.96$ * (X)
$\mathrm{T}=275.52$
$\mathrm{T}=276 \quad$ vehicle trips
with 50\% ( 138 vph ) entering and 50\% ( 138 vph ) exiting.

## SATURDAY Daily

$\mathrm{T}=291.67 *(\mathrm{X})$
$\mathrm{T}=3,500.04$
$\mathrm{T}=3,500 \quad$ vehicle trips
with $50 \%(1,750$ vpd) entering and $50 \%(1,750 \mathrm{vpd})$ exiting.

## Saturday Midday Peak Hour Of Generator

$\begin{aligned} \mathrm{T}= & 23.26 *(\mathrm{X}) \\ \mathrm{T}= & 279.12 \\ \mathrm{~T}= & 279 \quad \quad \text { vehicle trips } \\ & \text { with } 50 \%\left(\begin{array}{cc}140 & \text { vph }\end{array}\right) \text { entering and } 50 \%\left(\begin{array}{lll}139 & \text { vph }\end{array}\right) \text { exiting. }\end{aligned}$


[^0]:    ${ }^{\mathrm{a}}$ In miles per hour (mph).
    ${ }^{\mathrm{b}}$ Speed at, or below which 85 percent of all observed vehicles travel.

[^1]:    ${ }^{1}$ A Policy on Geometric Design of Highways and Streets; American Association of State Highway and Transportation Officials (AASHTO); 2009.

[^2]:    ${ }^{2}$ Drive-Through Queue Generation; Mike Spack, PE, PTOE; CountingCars.com; February 2012.

[^3]:    ${ }^{3}$ Trip Generation Manual, $10^{\text {th }}$ Edition; Institute of Transportation Engineers; Washington, DC; 2017.
    ${ }^{4}$ Trip Generation Handbook; $3^{r d}$ Edition; Institute of Transportation Engineers; Washington, DC; August 2014.

[^4]:    25 MPH
    30 MPH
    35 MPH
    38 MPH
    31 MPH
    $26-35 \mathrm{MPH}$
    2328
    $71.1 \%$
    1893
    $57.8 \%$ 15th Percentile :
    50th Percentile :
    85th Percentile : 85th Percentile
    95th Percentile
    
    

[^5]:    25 MPH
    30 MPH
    34 MPH
    38 MPH

