

56 Teresa Road Hopkinton, MA 01748 Tel.: (508) 395-1576 Fax: (508) 435-2481 www.RonMullerAssociates.com

Traffic Assessment

Gas Station/Convenience Store 23 Central Street Byfield, Massachusetts



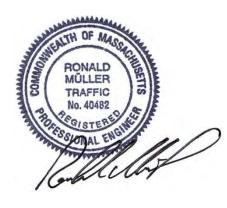
Accuracy

Prepared for:

A.L. Prime Energy Consultant, Inc. 18 Lark Avenue Saugus, MA 01906



Integrity



March 11, 2020





Traffic Assessment

To:	Mr. Anthony Guba	Reg:	Gas Station/Convenience Store
	A.L. Prime Energy Consultant, Inc.		23 Central Street
	18 Lark Avenue		Byfield, MA
	Saugus, MA 01906		
		Date:	March 11, 2020
From:	Kirsten Braun, P.E., Senior Traffic Eng.	Project #:	19046
	Ron Müller, P.E., Principal		

INTRODUCTION

Ron Müller & Associates (RMA) has conducted this Traffic Assessment for the development of a gas station and convenience store to be located at 23 Central Street in Byfield, Massachusetts. As proposed the project consists razing the single-family home on site and constructing a gasoline station with 12 vehicle fueling positions and an approximately 4,850 square-foot convenience store with drive-through window for a coffee/donut shop. Access will be provided via two curb cuts on Central Street. The site is located just east of exit 55 on Route I-95. The site location is shown in Figure 1.

This report has been prepared to estimate the project traffic generation, evaluate site access and on-site circulation, and provide a qualitative assessment of the impacts of this traffic on the adjacent streets. As documented in this report, the majority of traffic to/from the gas station and convenience store (and in particular the drive-through coffee/donut shop) is drawn from the traffic that already exists on the adjacent street or is diverted from I-95. Accordingly, traffic increases on Central Street are expected in the range of 37 to 38 additional vehicles during peak hours, or an average of one additional vehicle every 1 ½ to two minutes. This additional traffic represents an increase of 5 to 6 percent over existing volumes. The largest increase in traffic from the development project will occur on the short section of Central Street between the site and I-95, with 86 to 89 additional vehicles during the peak hours, or an average of one to two additional vehicles every minute.

The proposed driveway design is appropriate to accommodate both passenger and delivery vehicles and more than adequate sight distances will be provided in all directions to allow safe operation. It is recommended that all landscaping and signs are kept low to the ground or set

outside the sight triangles. The proposed drive-through lane can accommodate an anticipated average maximum queue length of 11 vehicles. The maximum potential queue of 13 vehicles can also be accommodated on-site without affecting circulation or access. It is recommended that the proposed drive-through lane be clearly identified through signing and striping.

Figure 1 Site Location Map



TRAFFIC VOLUMES AND VEHICLE SPEEDS

Traffic volume and vehicle speed information along Central Street just east of Fruit Street near the site was obtained from an automatic traffic recorder (ATR) count conducted in November 2019. To determine if the count data needed to be adjusted to represent annual average-month conditions, historical traffic volume data were obtained from the MassDOT. Based on the nearest MassDOT permanent count station located on Interstate 95, north of Topsfield (Station 5085), traffic during the month of November is approximately four percent below annual average-month conditions. Therefore, the November counts were increased by 4.0 percent. The traffic counts and MassDOT permanent count station data are provided in the Appendix. A summary of the available traffic counts is shown in Table 1. Additionally, a summary of observed travel speeds along Central Street are summarized in Table 2.

Table 1Existing Traffic Volume Summary

Location	Daily Volume ^a	Peak Volu	Hour 1me ^b	K-Factor ^c	Directional Distribution ^d
Central Street	7,250	AM:	733	10.1%	56% EB
East the Site:		PM:	646	8.9%	46% EB

^a In vehicles per day.

^b In vehicles per hour.

^c Percentage of daily traffic occurring during the peak hour.

 d NB = northbound, SB = southbound.

Table 2 Observed Travel Speeds ^a

Location/Direction	Posted	Average	85 th Percentile
	Speed Limit	Speed	Speed ^b
Central Street East of the site Eastbound Westbound	35 35	31 29	36 34

^a In miles per hour (mph).

^b Speed at, or below which 85 percent of all observed vehicles travel.

As shown, the average recorded speeds along Central Street adjacent to the site were slightly lower than the posted speed limit of 35 miles per hour (mph) with 31 mph eastbound and 29 mph westbound. Average speeds along this stretch of roadway may be slightly lower than the posted speed limit due to the proximity to the I-95 ramps. The 85th percentile speeds were recorded to be 36 mph eastbound and 34 mph westbound, comparable to the posted speed limit. 85th percentile speeds were accordingly used in the calculation of minimum sight distance requirements, as described below.

SIGHT DISTANCE

To identify potential safety concerns associated with site access and egress, sight distances have been evaluated at the proposed site driveways on Central Street to determine if the available sight distances for vehicles exiting the site meet or exceed the minimum distances required for approaching vehicles to safely stop. The available sight distances were compared with minimum requirements, as established by the American Association of State Highway and Transportation Officials (AASHTO).¹ AASHTO is the national standard by which vehicle sight distance is calculated, measured, and reported. The Massachusetts Department of Transportation (MassDOT) and the Executive Office of Energy and Environmental Affairs (EEA) require the use of AASHTO sight distance standards when preparing traffic impact assessments and studies, as stated in their guidelines for traffic impact assessments.

Sight distance is the length of roadway ahead that is visible to the driver. Stopping Sight Distance (SSD) is the minimum distance required for a vehicle traveling at a certain speed to safely stop before reaching a stationary object in its path. The values are based on a driver perception and reaction time of 2.5 seconds and a braking distance calculated for wet, level pavements. When the roadway is either on an upgrade or downgrade, grade correction factors are applied. Stopping sight distance is measured from an eye height of 3.5 feet to an object height of 2 feet above street level, equivalent to the taillight height of a passenger car. The SSD is measured along the centerline of the traveled way of the major road.

Intersection sight distance (ISD) is provided on minor street approaches to allow the drivers of stopped vehicles a sufficient view of the major roadway to decide when to enter the major roadway. By definition, ISD is the minimum distance required for a motorist exiting a minor street to turn onto the major street, without being overtaken by an approaching vehicle reducing its speed from the design speed to 70 percent of the design speed. ISD is measured from an eye height of 3.5 feet to an object height of 3.5 feet above street level. The use of an object height equal to the driver eye height makes intersection sight distances reciprocal (i.e., if one driver can see another vehicle, then the driver of that vehicle can also see the first vehicle). When the minor street is on an upgrade that exceeds 3 percent, grade correction factors are applied.

¹A Policy on Geometric Design of Highways and Streets; American Association of State Highway and Transportation Officials (AASHTO); 2009.

SSD is generally more important as it represents the minimum distance required for safe stopping while ISD is based only upon acceptable speed reductions to the approaching traffic stream. However, the ISD must be equal to or greater than the minimum required SSD in order to provide safe operations at the intersection. In accordance with the AASHTO manual, "If the available sight distance for an entering or crossing vehicle is at least equal to the appropriate stopping sight distance for the major road, then drivers have sufficient sight distance to anticipate and avoid collisions. However, in some cases, this may require a major-road vehicle to stop or slow to accommodate the maneuver by a minor-road vehicle. To enhance traffic operations, intersection sight distances that exceed stopping sight distances are desirable along the major road." Accordingly, ISD should be at least equal to the distance required to allow a driver approaching the minor road to safely stop.

The available sight distances at the proposed driveways on Central Street were measured and compared to minimum requirements as established by AASHTO. On Central Street, the 85th percentile speeds were used over the posted speed limit of 35 mph to determine minimum required sight distance. The required minimum sight distances are compared to the available distances, as shown in Table 3.

	Interse	ction Sight Distan	ce (feet)
Location/Direction	Measured	Minimum Required ^a	Desirable ^b
Central St. at East Site Driveway:			
East of intersection	410	250	390
West of Intersection	500+	261	390
Central St. at West Site Driveway:			
East of intersection	490	250	390
West of Intersection	500+	261	390

Table 3Sight Distance Summary

^a Values based on AASHTO SSD requirements for the 85th percentile speed of 36 mph on Central Street traveling eastbound and 35 mph posted speed limit westbound.

^b Values based on AASHTO ISD requirements for the posted speed limit of 35 mph on Central Street.

As shown in the table, both the minimum required and desirable sight distances are exceeded at the east and west site driveways on Central Street and safe operation of these driveways can therefore be expected. To ensure that maximum sight distances are maintained, it is recommended that any proposed landscaping or signs in the vicinity of the driveways be kept low (maximum two feet in height from street level), or set back sufficiently so as not to impede the available sight distances.

SITE ACCESS, CIRCULATION AND QUEUING

Access to the site is proposed via two new curb cuts on Central Street. Both driveways are proposed to be 30 feet-wide which allows for less mountable curbing and pavement outside of the defined driveways to accommodate delivery-vehicle access to/from the site, in particular fuel deliveries. The site plan includes a swept path analysis showing how fuel delivery vehicles can navigate the site and driveways under this design.

The drive-through lane will provide approximately 220 feet of storage allowing for a total of 11 cars to be queued at the drive-through window. Beyond the drive-through lane, an additional 6 cars could be queued on site before affecting site access. The roadway around the building with the drive-through queue will be a minimum of 24 feet wide allowing more than adequate room for a vehicle to by-pass the drive-through queue. Although there are parking spaces located in the rear of the site adjacent to the drive-through lane, these spaces will be restricted to employee parking so as to minimize any conflict with the drive-through operation.

Based on a published study² of drive-through queuing at a number of different land uses, the average maximum queue at coffee shops was found to be 11 vehicles and the 85th percentile maximum queue was 13 vehicles. The study was performed at six different coffee shops including four Starbucks and two Caribou coffee shops located in Minnesota and Kansas. More locally, drive-through queue studies have been performed at four different Dunkin' Donuts coffee shops located in Malden, Kingston, and Dracut, Massachusetts and in Milford, New Hampshire. The maximum observed queue at these locations was 13 vehicles during the weekday AM peak hour, six vehicles during the weekday PM peak hour, and 10 vehicles during the Saturday peak hour. A summary of the queue studies is provided in the Appendix.

Based on these studies, there is adequate queuing space to accommodate the average maximum queue of 11 vehicles. The maximum queue of 13 vehicles observed at any one site can also easily be accommodated on site without affecting on site circulation or site access. The drive-through lane should be clearly marked through signing and striping including DRIVE THRU pavement markings with pavement arrows.

² Drive-Through Queue Generation; Mike Spack, PE, PTOE; CountingCars.com; February 2012.

TRIP GENERATION

The traffic to be generated by the proposed gas station and convenience store was estimated using the 10th Edition of the Institute of Transportation Engineers (ITE) *Trip Generation Manual*³. The site plan prepared by A.L. Prime Energy Consultant, Inc. shows 12 fueling positions on site as well as a 4,850 square-foot convenience store with coffee/donut shop. Accordingly, ITE Land Use Code 960 (Super Convenience Market/Gas Station) was used in estimating the traffic generation characteristics of the project, as shown in Table 4. The trip generation calculations are provided in the Appendix.

Time Period	Total Trips ^a	Pass-by Trips ^b	New Trips
Weekday Daily	2,770	1,550	1,220
Weekday AM Peak Hour			
Enter	169	105	64
Exit	168	105	<u>63</u>
Total	337	210	127
Weekday PM Peak Hour			
Enter	138	77	61
Exit	<u>138</u>	77	<u>61</u>
Total	276	154	122

Table 4Trip Generation Summary

^a ITE Land Use Code 960 (Super Convenience Market/Gas Station) for 12 fueling positions.

^b ITE Trip Generation Handbook. Pass-by rate of 62% applied to weekday AM total trips

and a pass-by rate of 56% applied to weekday daily and PM total trips.

As shown in Table 4, the gas station and convenience store on Central Street is expected to generate 337 vehicle trips (169 in and 168 out) during the weekday AM peak hour and 276 vehicle trips (138 in and 138 out) during the weekday PM peak hour. Not all vehicle trips generated by the project, however, represent new trips. Studies have shown that gas stations with convenience stores generate more than half of their business from the traffic already present on the adjacent roadway. This traffic is referred to as pass-by trips. Based on data published in the ITE *Trip Generation Handbook*,⁴ an average of 56 to 62 percent of the total traffic generated by gas stations with convenience stores is typically pass-by traffic, depending on the time period. Therefore, while the

³ Trip Generation Manual, 10th Edition; Institute of Transportation Engineers; Washington, DC; 2017.

⁴ Trip Generation Handbook; 3rd Edition; Institute of Transportation Engineers; Washington, DC; August 2014.

total traffic generated by the project will be realized at the site driveways, the impact of that traffic on the adjacent streets is substantially less.

As shown in Table 5, with the influence of pass-by traffic, the actual volume of new traffic to be added to the adjacent streets is 127 trips during the weekday AM peak hour (64 entering and 63 exiting) and 122 trips during the weekday PM peak hour (61 entering and 61 exiting).

TRIP DISTRIBUTION

The distribution of new site traffic on the area roadways was based on population densities near the site as well as access from the regional highway network. Accordingly, approximately 70-percent of site traffic is expected on Central Street to and from the west with at least 40 percent to/from I-95 and 30-percent to and from the east. The distribution of pass-by traffic was based on existing travel patterns on Central Street.

TRAFFIC INCREASES

The proposed development project will result in increases in traffic on the study area roadways. Traffic-volume increases on Central Street to the east of the site and west of I-95 are expected in the range of 37 to 38 vehicles during the peak hours, or an average of one additional vehicle every $1\frac{1}{2}$ to two minutes. This additional traffic represents an increase of 5 to 6 percent over existing Central Street traffic. The largest increase in traffic from the development project will occur on the short section of Central Street between the site and I-95, with 86 to 89 additional vehicles during the peak hours, or an average of one to two additional vehicles every minute.

CONCLUSIONS

- The project entails the development of a 12-vehicle fueling position gas station and 4,850 square foot convenience store with coffee/donut shop and drive through window at 23 Central Street in Byfield, Massachusetts. Site access is proposed via two new curb cuts to be constructed on Central Street.
- The proposed gas station with convenience store and coffee shop is expected to generate an additional 337 vehicle trips during the weekday AM peak hour (169 entering and 168

exiting) and 276 vehicle trips during the weekday PM peak hour (138 entering and 138 exiting). These additional trips will be realized at the site driveway.

- More than half of these trips will already be present on the adjacent roadways and will not be new to the area. These trips are referred to as pass-by trips. Accordingly, the increase in traffic on Central Street east of the site and west of I-95 is expected in the range of 37 to 38 vehicles during the peak hours, or an average of one additional vehicle every 1 ½ to two minutes. This additional traffic represents an increase of 5 to 6 percent over existing Central Street traffic.
- In the short section of Central Street between the site and I-95, an increase in traffic of 86 to 89 additional vehicles is expected during the peak hours, averaging about one to two additional vehicles every minute during both the AM and PM peak hours.
- Two 30-foot wide driveways with mountable curbing are proposed to accommodate larger delivery vehicles entering the site.
- Drive-through queue studies at coffee/donut shops have shown an average maximum queue length of 11 vehicles with the maximum queue at any one site observed at 13 vehicles. Based on the latest site plan the drive-through will be able to accommodate 11 vehicles. Beyond this drive-through lane, there is more than adequate room on site to accommodate a maximum potential queue of 13 vehicles without affecting on-site circulation and access. It is recommended that the drive-through lane be clearly marked through signing and striping including DRIVE THRU pavement markings with pavement arrows.
- The minimum required sight distances as well as the desired sight distances are exceeded at both site driveways on Central Street and therefore safe operation can be expected.
- It is recommended that any proposed landscaping or signs in the vicinity of the driveways be kept low to the ground (less than two feet above street level) or set back sufficiently so as not to impede sight distances for drivers exiting the site.

APPENDIX

Traffic Counts and Vehicle Speed Data Seasonal Adjustment Data Drive-Through Queue Studies Trip Generation Calculations

ed Totals		Totals		В		Totals		/B		11/20/201	Start
Afternoo	Morning	Afternoon	Morning	Afternoon	Morning	Afternoon	Morning	Afternoon	Morning	Wed	Time
				39	2			53	1		12:00
				42	2			47	0		12:15
				42	2			48	0		12:30
34	10	166	9	43	3	182	1	34	0		12:45
				27	4			49	1		01:00
				39	1			47	0		01:15
				41	1			41	0		01:30
36	8	173	7	66	1	189	1	52	0		01:45
				50	0			52	4		02:00
				66	0			110	1		02:15
				56	0			81	0		02:30
53	6	230	1	58	1	308	5	65	0		02:45
				83	0			68	2 2		03:00
				64	2			67			03:15
				69	4			94	1		03:30
58	11	286	6	70	0	302	5	73	0		03:45
				87	0			63	1		04:00
				78	3			73	2		04:15
				62	1			95	4		04:30
59	22	292	6	65	2	302	16	71	9		04:45
				76	5			90	18		05:00
				65	13			84	18		05:15
				71	13			83	27		05:30
59	129	282	44	70	13	310	85	53	22		05:45
				57	22			47	43		06:00
				46	32			50	43		06:15
				60	50			51	48		06:30
38	355	211	171	48	67	176	184	28	50		06:45
				23	100			34	68		07:00
				32	138			24	96		07:15
				36	101			19	94		07:30
21	729	123	399	32	60	90	330	13	72		07:45
				33	48			21	88		08:00
				25	62			20	69		08:15
				32	52			12	52		08:30
18	482	107	214	17	52	74	268	21	59		08:45
				20	36			8	48		09:00
				18	30			9	49		09:15
				13	42			6	40		09:30
ç	320	63	139	12	31	34	181	11	44		09:45
				10	42			10	44		10:00
				8	35			3	64		10:15
				2	44			2	59		10:30
4	358	24	152	4	31	21	206	6	39		10:45
				8	27			3	39		11:00
				11	37			5	41		11:15
				5	35			0	44		11:30
3	304	27	142	3	43	9	162	1	38		11:45
398	2734			1984	1290			1997	1444		Total
59.3	40.7%			60.6%	39.4%			58.0%	42.0%		Percent

19024VL1

Start	11/21/201		/B		Totals		В		Totals		ed Totals
Time	Thu	Morning	Afternoon	Morning	Afternoon	Morning	Afternoon	Morning	Afternoon	Morning	Afternoo
12:00		0	44			5	38				
12:15		0	41			3	35				
12:30		1	48			1	39				
12:45		0	32	1	165	2	51	11	163	12	32
01:00		0	36			2	44				
01:15		1	47			2	41				
01:30		1	54			0	36				
01:45		0	45	2	182	0	64	4	185	6	36
02:00		1	54	2	102	1	56	-	100	Ū	00
02:00		0	101			1	73				
02:10		0	84			1	46				
02:45		0	70	1	309	0	64	3	239	4	54
				I	309			5	239	4	54
03:00		0	84			1	71				
03:15		5	84			1	77				
03:30		3	110	0	000	2	59	-	000	45	
03:45		0	88	8	366	3	73	7	280	15	64
04:00		2	91			0	70				
04:15		3	85			3	60				
04:30		2	84			0	68	-			
04:45		13	74	20	334	6	62	9	260	29	59
05:00		15	113			6	60				
05:15		23	103			4	70				
05:30		25	84			13	83				
05:45		20	56	83	356	28	80	51	293	134	64
06:00		48	59			20	63				
06:15		30	41			27	47				
06:30		57	26			41	54				
06:45		62	31	197	157	69	57	157	221	354	37
07:00		55	33			79	49				
07:15		62	25			129	42				
07:30		111	32			89	65				
07:45		62	25	290	115	94	29	391	185	681	30
08:00		73	16	200		113	34	001	100	001	00
08:15		61	25			74	40				
08:30		67	20			55	30				
08:45		58	16	259	77	65	31	307	135	566	21
08.45		51	20	259		52	20	307	155	500	21
09:00		57	14			52	33				
09:30		55	16	226	80	45 40	20	190	00	416	17
09:45		63	30	220	80		17	190	90	410	17
10:00		65	8			33	13				
10:15		47	8			36	9				
10:30		37	5	105		29	9				_
10:45		44	16	193	37	47	8	145	39	338	7
11:00		42	5			41	7				
11:15		51	6			39	7				
11:30		53	1			45	5				
11:45		46	3	192	15	40	7	165	26	357	4
Total		1472	2193			1440	2116			2912	430
Percent		40.2%	59.8%			40.5%	59.5%			40.3%	59.7
Grand		2916	4190			2730	4100			5646	829
Total		2910	4190			2130	4100			3040	029
		41.0%	59.0%			40.0%	60.0%			40.5%	59.59
Percent											

Location : Central Street Location : East of Fruit Street City/State: Byfield, MA

19024VL1

Page 1

Time WB EB WB EB WB EB WB EB WB 11 01:00 AM \times \times \times \times \times 1 1 01:00 AM \times \times \times \times \times 1 1 02:00 A \times \times \times \times \times 1 1 03:00 \times \times \times \times \times 1 1 07:00 \times \times \times \times \times 1 1 07:00 \times \times \times \times \times 2 3 07:00 \times \times \times \times \times 2 3 08:00 \times \times \times \times 2 3 3 01:00 \times \times \times \times 2 3 3 01:00 \times \times \times \times 2 3 3 01	EB						,
	σ	WB EB	WB EB	WB EB	WB EB	WB EB	ല
	,	1	*	*	*	~	10
1 1	7	2	*	*	*	7	9
1 1	~	1	*	*	*	ო	2
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	9	8 7	*	*	*	9	9
1 1		20 9	*	*	*	18	8
1 1		4,	*	*	*	84	48
1 1			*	*	*	190	164
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$		290 391	*	*	*	310	395
$\begin{array}{cccccccccccccccccccccccccccccccccccc$			*	*	*	264	260
1 1		226 190	*	*	*	204	164
10 10 11 12 14 15 14 14 14 14 14 14 14 14 14 14			*	*	*	200	148
$\begin{array}{cccccccccccccccccccccccccccccccccccc$			*	*	*	177	154
10 10 11 12 14 14 14 14 14 14 14 14 14 14			*	*	*	174	164
			*	*	*	186	179
			*	*	*	308	234
			*	*	*	334	283
3 3 3 4 4 4 4 5 4 5 5 5 5 5 5 5 5			*	*	*	318	276
17 17 17 17 17 17 17 17 17 17			*	*	*	333	288
			*	*	*	166	216
* * * * * * * * * * * * * * * * * * *			*	*	*	102	154
* * * 3 * * * * 2 * * * 0 0 2 * * * 2 344 0 07:0 17:0			*	*	*	76	121
* * * 2 * * * * 2 * 0 0 344 2 * 0 0 0 344 - - - 0 333 - - - - 333 - - - - 170			*	*	*	57	76
* 333 33 33 33 33 33 33 3			*	*	*	29	32
/ 0 0 0 0 344 / 0 0 0 - 347 07:0 33			*	*	*	12	26
/ 0 0 0 0 - 07:0 07:0 33		(T)	0	0	0	3554	3414
· · · ·	6715	7221	0	0	0	6968	
• •	0			•	•		02:00
			•	•	•		395
	16:00	15:00 17:00	•	•	•	15:00	17:00
•		366 293				334	288
Comb. 0 0 for the combined of	6715	7221	0	0	0	6968	
ADT ADT 6,968 AADT 6,968							

$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	$ \begin{array}{cccccccccccccccccccccccccccccccccccc$
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	$ \begin{array}{cccccccccccccccccccccccccccccccccccc$
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	$ \begin{array}{cccccccccccccccccccccccccccccccccccc$
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	$ \begin{array}{cccccccccccccccccccccccccccccccccccc$
	$ \begin{array}{cccccccccccccccccccccccccccccccccccc$
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	$ \begin{array}{cccccccccccccccccccccccccccccccccccc$
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	$ \begin{array}{cccccccccccccccccccccccccccccccccccc$
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	$\begin{bmatrix} 1 \\ 2 \\ 2 \\ 2 \\ 2 \\ 2 \\ 2 \\ 2 \\ 2 \\ 2 \\$
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	$ \begin{bmatrix} 12 \\ 12 \\ 12 \\ 13 \\ 14 \\ 15 \\ 15 \\ 15 \\ 15 \\ 15 \\ 15 \\ 15$
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	$ \begin{bmatrix} 120 \\ 1$
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	1 1
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	$ \begin{array}{cccccccccccccccccccccccccccccccccccc$
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	$ \begin{array}{cccccccccccccccccccccccccccccccccccc$
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	$\begin{bmatrix} 2 & 2 & 2 \\ 2 & 3 & 3 \\ 3 & 3 & 3 & 3 \\ 3 & 3 & 4 & 3 \\ 3 & 3 & 4 & 3 \\ 3 & 3 & 4 & 3 \\ 3 & 3 & 4 & 3 \\ 3 & 3 & 4 & 3 \\ 3 & 4 & 4 & 1 \\ 1 & 1 & 1 & 1 \\ 1 & 1 & 1 & 1 \\ 3 & 1 & 1 & 1 \\ 3 & 1 & 1 & 1 \\ 3 & 1 & 1 & 1 \\ 1 & 1 & 1 & 1 \\ 1 & 1 & 1$
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	$\begin{array}{cccccccccccccccccccccccccccccccccccc$
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	$\begin{array}{cccccccccccccccccccccccccccccccccccc$
78 30 86 18 74 1 13 4 14 4 15 4 16 0 17 1 18 0 19 1 10 0 11 1 12 1 120 1 121 1	78 88 74 88 88 88 88 88 88 88 89 89 89 89 13 14 <td< td=""></td<>
86 18 1 44 17 1 0 0 0 0 133 4 4 2 0 0 0 0 0 1 1 1 1 0 0 0 0 0 0 0 1 1 1 1 0	86 18 1 0
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	$\begin{array}{cccccccccccccccccccccccccccccccccccc$
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	$ \begin{array}{cccccccccccccccccccccccccccccccccccc$
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	
	TIΘ%Φ

Location : Central Street Location : East of Fruit Street City/State: Byfield, MA	et Street					COCZ-400-016	0							19024SP1
			26	31	36	41	46	51	56	61	66	71	76	
1	15 2	20 25	30	35	40	45	50	55	60	65	70	75	666	Total
	0 0	0,		0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	c
		- c	- c	⊃ ←	00			- c						v ←
	0		0	0	0	0	0	0	0	0	0	0	0	8
	0 0		31	9 22	4 0	0 -	00	00	0 0	0 0	0 0	00	00	20
	0 0		5 12	79	17		00	00	00	00	00	00	00	00 197
	0 01		91	101	40	5	0	0	0	0	0	0	0	290
	ر (8 32	2 <u>0</u>	97 	39 31	91	0 ·	0	0	0 (0	0	0	259
	0 0		87 66	76	37	n D	- c	00	0 0	0 0	0 0	00	00	226
	- C		20 28	68 68	29 29	מיס		00				00		193
	- 0		57	64	202	o ←	0	00	0	0	0	0	0	165
	0		58	20	29	5	0	0	0	0	0	0	0	182
	0		101	113	47	12	0	0	0	0	0	0	0	309
	0		148	120	43	ں م	0	. -	0	0	0	0	0	366
	- 0		122	115	27	، ک	0 0	0 0	0 0	0 0	0 0	0 0	0 0	334
	ი ი	7 688	142	113	23		0 0	00	0 0	0 0	0 0	00	00	356
			4 5	70 80	- u	- c								115
	0	2 9	36	23 1	9	• ~	0	0	0	0	0	0	0	11
	-	2	28	29	6	0	0	0	0	0	0	0	0	80
	0 0		9 0	52	4,	0 (0 0	0 0	0 0	0 0	0 (0 0	0 0	37
	0 0	0 3 105 461	3 1311	1290	1 427	0 0	0 +	0 +				00		15 3665
				0071	175	8	_	_	þ	þ	þ	5	Þ	0000
		15th Percentile 50th Percentile 85th Percentile 95th Percentile		24 MPH 29 MPH 34 MPH 38 MPH										
	Me	Mean Speed(Average)		30 MPH										
	2	Number in Pace	· · ·	2601										
	Number of \	/ehicles > 30 MPH		71.0% 1779										
	Percent of V	Percent of Vehicles > 30 MPH		48.5%										
	17 2	218 932	2511	2495	807	119	9	۲	0	0	0	0	0	7106
		15th Percentile		MPH										
		50th Percentile 85th Percentile		29 MPH 34 MPH										
		95th Percentile		НЧМ										
	Me 10	Mean Speed(Average) 10 MPH Pace Speed Number in Pace		30 MPH 35 MPH 5006										
	Number of \ Percent of \	Percent in Pace Number of Vehicles > 30 MPH Percent of Vehicles > 30 MPH		70.4% 3428 48.2%										

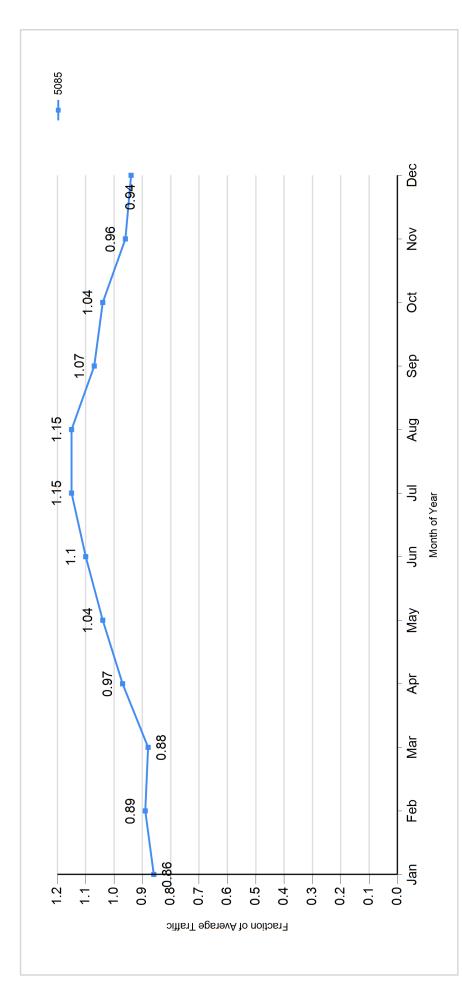
Location : Central Street	d Street) -)							
City/State: Byfield, MA EB	d, MA														19024SP1
Start	~	16	21	26	31	36	41	46	51	56	61	99	71	76	
Time	15	20	25	30	35	40	45	50	55	60	65	70	75	666	Total
11/20/19	0	0	0	4	5	0	0	0	0	0	0	0	0	0	б
01:00	0	2	0	-	ო		0	0	0	0	0	0	0	0	7
02:00	0	0	0	-	0	0	0	0	0	0	0	0	0	0	-
03:00	0	0		-	ო		0	0	0	0	0	0	0	0	9
04:00	0	0	2	2	-	0	0	-	0	0	0	0	0	0	9
05:00	0	0	7	16	23	с	0	0	0	0	0	0	0	0	44
00:90	0	-	ø	42	83	33	e	-	0	0	0	0	0	0	171
00:20	ო	-	19	06	194	79	11	7	0	0	0	0	0	0	399
08:00	2	4	15	48	96	40	7	2	0	0	0	0	0	0	214
00:60	0	5	4	38	62	27	ო	0	0	0	0	0	0	0	139
10:00	9	8	17	29	59	28	e	-	-	0	0	0	0	0	152
11:00	-	-	15	39	65	20	0	-	0	0	0	0	0	0	142
12 PM	0	ო	20	42	66	28	7	0	0	0	0	0	0	0	166
13:00	0	7	18	48	64	31	4	~	0	0	0	0	0	0	173
14:00	5	8	20	76	86	30	4	0	0	-	0	0	0	0	230
15:00	7	4	33	86	110	50	-	0	0	0	0	0	0	0	286
16:00	4	10	36	06	121	26	4	0	0	0	0	-	0	0	292
17:00	0	11	32	94	109	33	e	0	0	0	0	0	0	0	282
18:00	0	7	19	96	70	19	0	0	0	0	0	0	0	0	211
19:00	0	4	12	55	47	4	£-	0	0	0	0	0	0	0	123
20:00	0	9	17	34	42	7	-	0	0	0	0	0	0	0	107
21:00	0	-	5	23	28	9	0	0	0	0	0	0	0	0	63
22:00	0	0	4	ø	ø	ო	~	0	0	0	0	0	0	0	24
23:00	0	-	2	10	10	ო	-	0	0	0	0	0	0	0	27
Total	23	84	301	973	1355	472	54	6	-	-	0	-	0	0	3274
Daily		15th 50th 85th 95th	15th Percentile : 50th Percentile : 85th Percentile : 95th Percentile :	25 MPH 30 MPH 35 MPH 38 MPH	IIII										
					-										
	Numl	Mean Speed(Average) 10 MPH pace Speed Number in Pace Percent in Pace Number of Vehicles > 30 MPH Percent of Vehicles > 30 MPH	Ppea(Average) : PH Pace Speed : Number in Pace : Percent in Pace : inles > 30 MPH : incles > 30 MPH :	31 MPH 26-35 MPH 2328 71.1% 1893 57 8%	L T 83 % 65 %										
	-				2										

Page 4	19024SP1	Total 11 11 145 157 145 145 145 165 165 165 165 165 165 185 185 185 185 185 185 1355 30 223 30 26 33 33 26 33 33 3556	6830	
		4 000000000000000000000000000000000000	-	
		7 7	0	
		90000000000000000000000000000000000000	4	
		0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	e	
			1	
		00000000000000000000000000000000000000	0	
Accurate Counts 978-664-2565		44 45 46 6 6 6 6 6 6 6 	135 20	
Accur 978		36 36 40 4 40 40 4 40 4 42 42 42 42 43 42 42 40 44 42 42 42 42 42 42 42	1143 1:	
		35 37 35 37 37 37 37 37 37 37 37	2864 11	
		26 30 33 33 35 58 58 58 58 58 58 58 58 58 58 58 58 58	1889 2	25 MPH 31 MPH 36 MPH 39 MPH 31 MPH 26-35 MPH 4753 69.6% 61.1%
		21 25 0 0 0 0 16 11 14 11 14 14 14 14 14 14 14 14 14 14	581	centile : centile : centile : centile : centile : centile : cerage) : Prace : n Pace : n Pace : n MPH :
		16 21 20 25 20 25 1 0 0 0 1 1 2 1 2 2 2 2 1 1 2 1 3 1 3 1 3 1 3 1 3 1 3 1 3 1 3 2 3 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 </td <td>146</td> <td>15th Percentile : 50th Percentile : 85th Percentile : 85th Percentile : 95th Percentile : Mean Speed(Average) : 10 MPH Pace Speed : Number in Pace : Percent in Pace : Number of Vehicles > 30 MPH : Percent of Vehicles > 30 MPH :</td>	146	15th Percentile : 50th Percentile : 85th Percentile : 85th Percentile : 95th Percentile : Mean Speed(Average) : 10 MPH Pace Speed : Number in Pace : Percent in Pace : Number of Vehicles > 30 MPH : Percent of Vehicles > 30 MPH :
	Street ⁷ ruit Street MA	7. 1 Number Percent	41	Number Percent
	Location : Central Street Location : East of Fruit Street City/State: Byfield, MA FB	EB Start 11/21/19 01:00 02:00 02:00 03:00 04:00 07:00 07:00 07:00 07:00 11:00 12 PM 14:00 12 PM 14:00 17:00 17:00 17:00 12 PM 12 PM	Grand Total	Overall

	19024SP1		Total	10	ω	9	11	22	129	355	729	482	320	358	304	348	362	538	588	594	592	387	213	181	97	45	36	6715					
		76	666	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0					
		71	75	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0					
		99	20	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	-	0	0	0	0	0	0	0	۰					
		61	65	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0					
		56	09	0	0	0	0	0	0	0	0	0	0	0	0	0	0	-	0	0	0	0	0	0	0	0	0	-					
		51	55	0	0	0	0	0	0	0	0	0	0	-	0	0	0	0	0	0	0	0	0	0	0	0	0	-					
5		46	50	0	0	0	0	2	0	2	2	e	0	2	~	0		0		0	0	0	0	0	0	0	0	14					
		41	45	-	0	0	0	0	4	7	16	10	9	6	4	7	ω	6	б	10	4	-	ო	.	0	2	2	113					
		36	40	0	-	-	~	ю	17	64	127	78	42	42	35	53	54	59	66	56	51	36	8	11	7	ო	4	852					
		31	35	5	ო	2	4	7	66	164	323	216	134	147	129	131	126	190	203	199	195	114	17	60	39	15	11	2560		- -	F 00 vo	0.0	
		26	30	4	-	-	4	5	35	89	182	124	103	89	89	105	117	185	191	207	240	176	93	67	33	19	14	2173	25 MPH 30 MPH 34 MPH 38 MPH	31 MPH	70.535 MPT 473 70.5%	3542 52.7%	
		21	25	0	0	-	2	5	5	22	66	38	24	46	39	41	37	67	68	95	87	44	26	34	16	9	ო	772	15th Percentile : 50th Percentile : 85th Percentile : 95th Percentile :	Average) :	РН Расе Speed : Number in Pace : Percent in Pace :	30 MPH : 30 MPH :	
		16	20	0	ო	0	0	0	7	7	10	11	1	15	4	10	19	22	15	21	14	15	9	80	2	0	2	197	15th F 50th F 85th P 95th F	Mean Speed(Average)	10 IMPH Pace Speed Number in Pace Percent in Pace	Number of Vehicles > 30 MPH Percent of Vehicles > 30 MPH	
Street Truit Street	MA	~	15	0	0	-	0	0	0	0	ო	2	0	7	ო	-	0	5	2	5	-	-	0	0	0	0	0	31				Numbe Percen	
Location : Central Street Location : Fast of Emit Street	City/State: Byfield, MA WB, EB	Start	Time	11/20/19	01:00	02:00	03:00	04:00	02:00	00:90	07:00	08:00	00:60	10:00	11:00	12 PM	13:00	14:00	15:00	16:00	17:00	18:00	19:00	20:00	21:00	22:00	23:00	Total	Daily				

Page 6	19024SP1	71 76 75 999 Total 0 0 12 0 0 12 0 0 14 0 0 14 0 0 14 0 0 134 0 0 134 0 0 134 0 0 134 0 0 134 0 0 134 0 0 134 0 0 134 0 0 134 0 0 134 0 0 134 0 0 11 0 0 11 0 0 170 0 0 170 0 0 170 0 0 170 0 0 170 0 0 170 0 <	0 1 13936	
		900000000000000000000000000000000000000	~	
		60 00000000000000000000000000000000000	m	
		00000000000000000000000000000000000000	~	
		Ω Ω	۵	
Counts -2565		4 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	26	
Accurate Counts 978-664-2565		4 4 0 0 0 0 0 - 9 9 7 8 7 7 8 9 9 9 4 8 8 8 9 0 4 8 8 8 9 9 7 8 8 9 9 7 8 8 9 9 7 8 8 8 9 9 7 8 8 8 8	254	
		$\begin{array}{c} 36 \\ 76 \\$	1950	
		26 31 30 35 2 4 6 1 2 3 2 4 6 1 2 31 1 2 35 1 4 6 53 41 156 11 277 147 147 277 166 147 277 166 95 118 271 101 154 273 111 154 273 111 116 116 111 118 111 111 118 111 111 111 111 111 111 111 111 113 36 36 MPH 30 37 MPH 36026 65026 65026 65026 65026 6508 56.2% 56.2% 56.2% <td>400 5359 25 MPH 36 MPH 38 MPH 38 MPH 38 MPH 31 MPH</td> <td>54.5%</td>	400 5359 25 MPH 36 MPH 38 MPH 38 MPH 38 MPH 31 MPH	54.5%
			4400	
		16 21 20 25 0 0 0 1 0 0 1 0 0 1 1 1 1 25 27 1 1 27 1 2 7 1 3 55 1 13 55 1 33 56 1 13 55 1 4 44 14 6 33 15 86 53 14 6 33 15 86 54 16 1 5 16 1 4 16 1 4 16 1 4 16 56 5 16 1 4 16 1 4 16 1 4 16 1 4 <	364 1513 15th Percentile: 50th Percentile: 85th Percentile: 95th Percentile: 95th Percentile: 95th Percentile:	Number in Pace : Number in Pace : Percent in Pace : hicles > 30 MPH : hicles > 30 MPH :
	eet	16 21 20 25 20 25 0 0 0 1 0 1 1 1 21 20 25 27 1 1 1 2 3 5 3 5 6 11 27 7 12 7 23 13 5 55 14 4 33 15 4 33 15 86 37 16 1 29 17 4 27 18 4 27 19 1 4 167 1 4 167 1 24 167 1 24 167 1 4 167 1 4 167 1 4 167 1 4	364 Mean S	Number of Vehicles > 30 Percent in Percent of Vehicles > 30
ntral Street	st of Fruit Str field, MA	- ΰ 00000-00ωω0-40-0-00 ² Σπ	28	Ζü
Location : Central Street	Location : East of Fruit Street City/State: Byfield, MA WR FR	WB, EB Start Time 11/21/19 01:00 05:00 05:00 05:00 03:00 04:	Grand Total Overall	

Massachusetts Highway Department



Traffic Pattern by Month for 1/1/2018 - 12/31/2018

Traffic Pattern by Month for 1/1/2018 - 12/31/2018

Factor Group	Station	Weight	Jan	Feb	Mar	Apr	May	Jun	Jul	Aug	Sep	Oct	Nov	Dec
U1-Essex	5085	-	0.862	0.891	0.878	0.972	1.039	1.098	1.149	1.151	1.072	1.044	0.963	0.938
	Average of Weighted Factors	l Factors	0.862	0.891	0.878	0.972	1.039	1.098	1.149	1.151	1.072	1.044	0.963	0.938

Observed Vehicle Queues at Dunkin Donut Drive-Through Lanes

Peak Hour/Queue	Sep-97 Malden, MA ^a	May-02 Milford, NH ^b	Sep-10 Kingston, MA ^c	Jul-16 Dracut, MA ^d	Average	Maximum
Weekday AM:						
Maximum	11	11	11	13	12	13
Average	9	4	5	6	6	9
Weekday PM:						
Maximum	6	3	4		4	6
Average	4	1	1		2	4
Saturday Midday:						
Maximum	8	10			9	10
Average	6	2			4	6

^aContains a Dunkin' Donuts with a drive-through window located in Adams Plaza in Malden, MA.

^bContains a Dunkin' Donuts with a drive-through window located at 143 Elm Street in Milford, NH.

^cContains a Dunkin Donuts with a drive-through window located on Routes 53/3A (Summer Street) in Kingston, MA.

^dContains a Dunkin Donuts with a drive-through window located at 177 Broadway Road in Dracut, MA.

Institute of Transportation Engineers (ITE); 10th Edition Land Use Code (LUC) 960 - Super Convenience Market/Gas Sation

Average Vehicle Trips Ends vs:Vehicle Fueling PositionsIndependent Variable (X):12

AVERAGE WEEKDAY DAILY

T = 230.52 * (X) T = 2,766.24 T = 2,770 vehicle trips with 50% (1,385 vpd) entering and 50% (1,385 vpd) exiting.

WEEKDAY MORNING PEAK HOUR OF ADJACENT STREET TRAFFIC

T = 28.08 * (X) T = 336.96 T = 337 vehicle trips with 50% (169 vph) entering and 50% (168 vph) exiting.

WEEKDAY EVENING PEAK HOUR OF ADJACENT STREET TRAFFIC

 $\begin{array}{l} T = 22.96 * (X) \\ T = 275.52 \\ T = 276 \\ \text{with } 50\% \ (138 \ \text{vph}) \ \text{entering and } 50\% \ (138 \ \text{vph}) \ \text{exiting.} \end{array}$

SATURDAY DAILY

T = 291.67 * (X) T = 3,500.04 T = 3,500 vehicle trips with 50% (1,750 vpd) entering and 50% (1,750 vpd) exiting.

SATURDAY MIDDAY PEAK HOUR OF GENERATOR

T = 23.26 * (X) T = 279.12 T = 279 vehicle trips with 50% (140 vph) entering and 50% (139 vph) exiting.